

## EXAMPLE 17

# Unsteady Flow Application

### Purpose

This example demonstrates the use of HEC-RAS to perform an unsteady flow analysis on a network of reaches containing junctions, storage areas, bridges, culverts, and hydraulic connections. The geometric data constructed for this example illustrates multiple unsteady flow applications in HEC-RAS. The geometric data used in this example is not a specific reference to any existing stream network.

This example focuses on modeling and evaluating storage areas, hydraulic connections, and hydraulic parameters in an unsteady flow environment. The user is referred to previous examples for discussion on modeling bridges, culverts, multiple reaches, and junctions.

To review the data files for this example, from the main program window select **File** and then **Open Project**. Select the project labeled “Unsteady Flow Application.” This will open the project and activate the following files:

Plan:	“Diamond River Base Plan”
Geometry:	“Diamond River Base Geometry”
Unsteady Flow:	“Unsteady Flow”

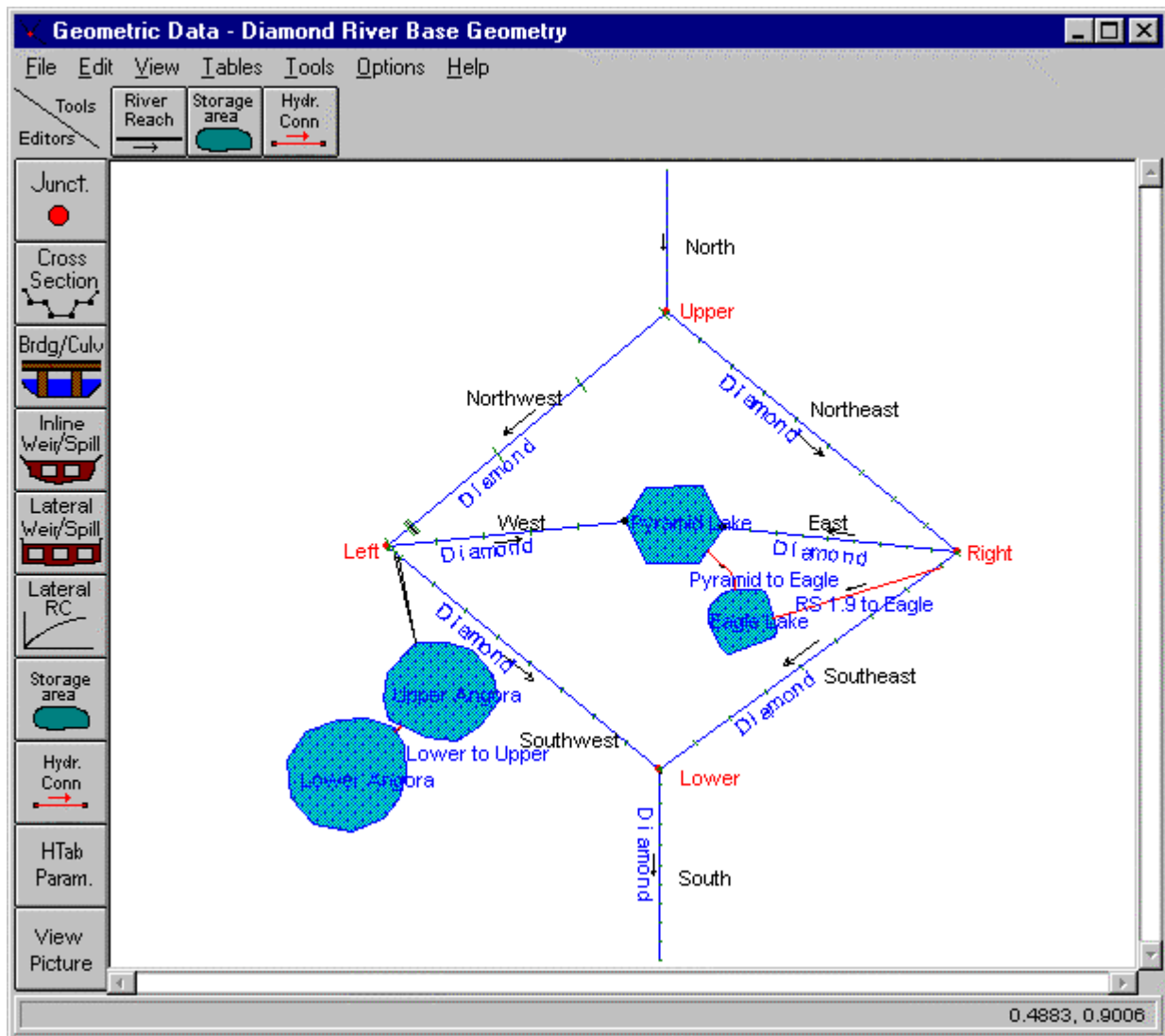
### Geometric Data

To view the geometric data for the river system, from the main program window select **Edit** and then **Geometric Data**. This will activate the **Geometric Data Editor** and display the river system schematic as shown in Figure 17.1.

### General Description

The Geometric Data consists of a network of channels connected by junctions. Four storage areas are included in the geometry (Pyramid Lake, Eagle Lake, Upper Angora, and Lower Angora). Pyramid Lake is connected by two reaches (East and West) and by Eagle Lake, via a culvert and a weir. Eagle Lake is hydraulically connected to River Station 1.9 on the Southeast Reach and to the previously mentioned Pyramid Lake. Upper Angora is connected to the Southwest Reach via a lateral weir located between cross sections 1.99 and

1.9. Lower Angora is hydraulically connected to Upper Angora via a culvert and a weir.



**Figure 17.1 River System Schematic for Diamond River Base Geometry**

In addition to these features, a culvert is located at River Station 3.395 on the Northwest Reach and a bridge is located at River Station 1.895 on the South Reach. The cross section geometry is prismatic with little change in elevation with distance except for the North Reach. The level topography provides an excellent environment to examine unsteady flow for such aspects as flow reversal.

This example assumes that the user has sufficient experience constructing the following river system features and all attributes associated with each feature.

- Reaches
- Cross Section Geometry

- Junctions
- Bridges and Culverts

Consult with previous examples for information pertaining to the above mentioned features. The following description commences with the creation of storage areas.

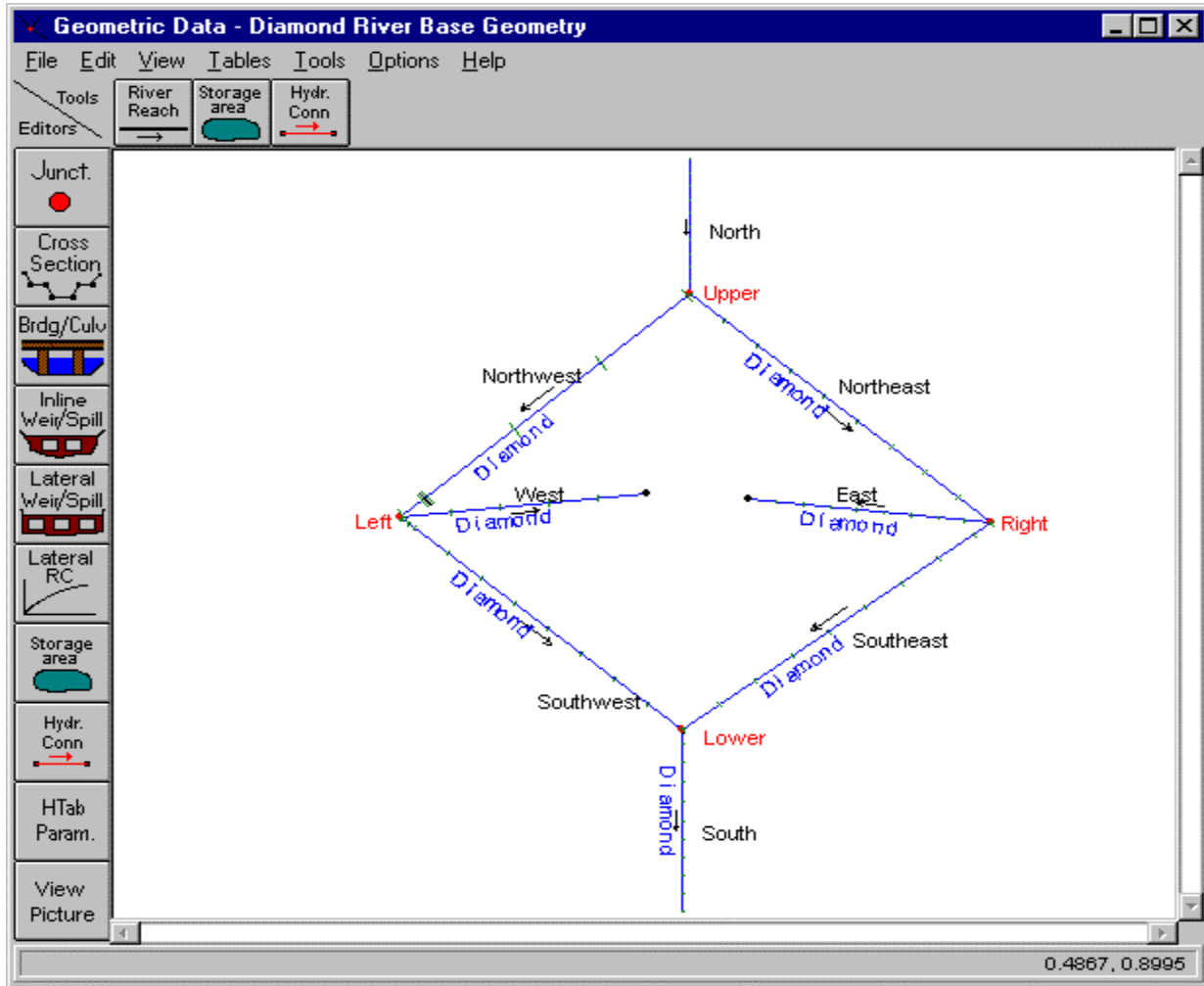


Figure 17.2 Initial Geometry for Development

## Creating Storage Areas

In the **Geometric Data Editor**, the starting geometry for this example is shown in Figure 17.2. On the **Tools** toolbar in the **Geometric Data Editor** the icon for **Storage area** was selected. The pointer was then placed between the East and West reaches and the storage area was drawn by single clicking and dragging the mouse for each desired point for the storage area. The final point for the storage area was selected by double clicking the mouse. Upon double clicking the mouse, a dialog box appears requesting a storage area name. The name “Pyramid Lake” was entered. Points on a storage area may

be added or removed by using the **Add Points to a Reach or SA** and **Remove Points to a Reach or SA** options found in the **Edit** menu.

It is important that the program recognizes the connection of Pyramid Lake to the East and West reaches. The connection may be made in two different ways. The first option is to draw the storage area so the end of each reach is included within the storage area polygon. The second option is to drag and drop the end of each reach within the storage area polygon by using the **Move Object** tool found in the **Edit** menu of the **Geometric Data Editor**. In both cases the reach and storage area are connected if a black dot appears at the intersection of the reach and the side of the storage area.

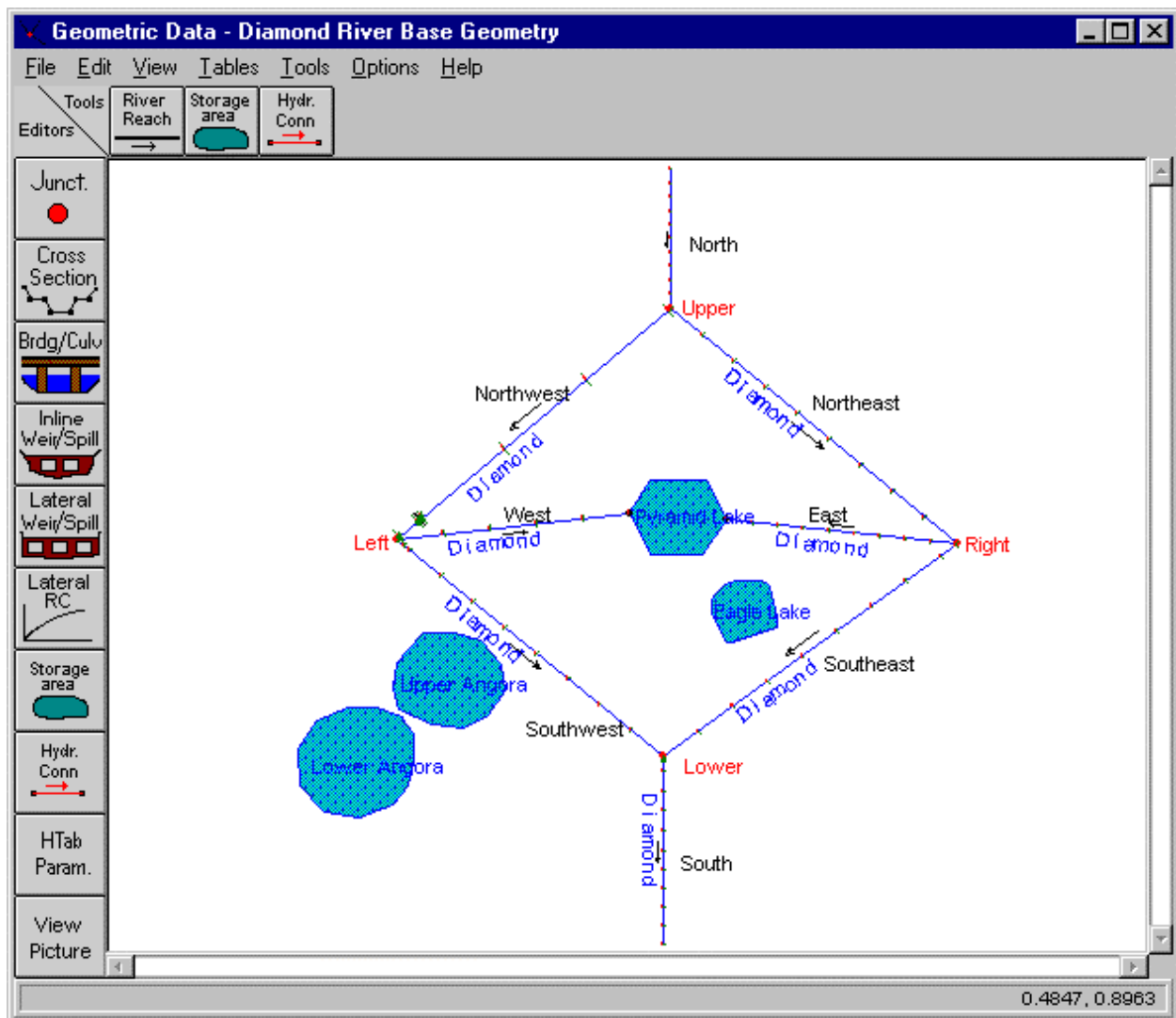


Figure 17.3 Geometry with Storage Areas

The remaining three storage areas were created in the same manner as Pyramid Lake without any connections to reaches. Figure 17.3 displays the

completed storage areas in the **Geometric Data Editor** before the addition of the hydraulic connections. The hydraulic connections for each storage area are discussed in impending sections.

## Entering Data for a Storage Area

After the storage area was drawn in the **Geometric Data Editor** the surface area for storage was entered. The data was entered in the **Storage Area Editor**, displayed in Figure 17.4, found by selecting the **Storage area** icon on the **Editors** toolbar. For Pyramid Lake the **Area times depth method** was selected. An area of 1000 acres with a minimum elevation of 0 feet was entered. This method computes the product of a constant area and change in water surface elevation to calculate storage volume.

**Storage Area Editor**

Storage Area: **Pyramid Lake** [v] [down] [up]

☒ Area times depth method    Area (acres):     Min Elev:

☐ Elevation versus Volume Curve

**Elevation Volume Curve**  
First elevation must have zero volume

	Elevation	Volume (ac-ft)
1		
2		
3		
4		
5		
6		
7		
8		
9		

OK    Cancel

**Figure 17.4 Storage Area Editor for Pyramid Lake**

An additional option for calculating storage volume is the **Elevation versus volume curve** method, shown in Figure 17.5 for Eagle Lake. For this method the initial elevation was entered as 0 feet with a volume of 0 acre-feet. Data was then entered in increments of 2 feet for the cumulative volume of storage produced at each increment. It is important to note that the user is not entering the surface area of storage at each increment but the cumulative volume of storage at each increment.

**Storage Area Editor**

Storage Area: Eagle Lake

☐ Area times depth method    Area (acres): 1000  
Min Elev: 0

☒ Elevation versus Volume Curve

**Elevation Volume Curve**  
First elevation must have zero volume

	Elevation	Volume (ac-ft)
1	0	0
2	2	1000
3	4	3000
4	6	5000
5	8	7000
6	10	10000
7	12	13000
8	14	16000
9	16	19000

OK    Cancel

Figure 17.5 Storage Area Editor for Eagle Lake

## Lateral Weir Connected to a Storage Area

A lateral weir was placed on the Southwest Reach by selecting the **Lateral Weir/Spillway** icon from the **Editors** toolbar in the **Geometric Data Editor**.

After selecting the icon, the **Lateral Weir Data Editor** appears, as shown in Figure 17.6. With the reach selected as Southwest, under the **Options** menu, **Add a Lateral Weir** was selected. The cross section location of the weir was entered at 1.95. This placed the weir between river stations 1.99 and 1.9.

To enter data for the weir, the **Weir/Embankment** icon was selected from the **Lateral Weir Data Editor**. The data was entered as shown in Figure 17.7 on the **Lateral Weir Embankment Window**. The weir is broad crested with a width of 10 feet, placed 10 feet from the upstream river station 1.99. The weir flow reference method was chosen as “water surface.”

Returning to the **Lateral Weir Data Editor**, the “right overbank” was selected from the pull-down position menu. Lastly, a connection was made to the storage area by selecting **Connect to a Storage Area**. On the pull-down menu “Upper Angora” was selected.

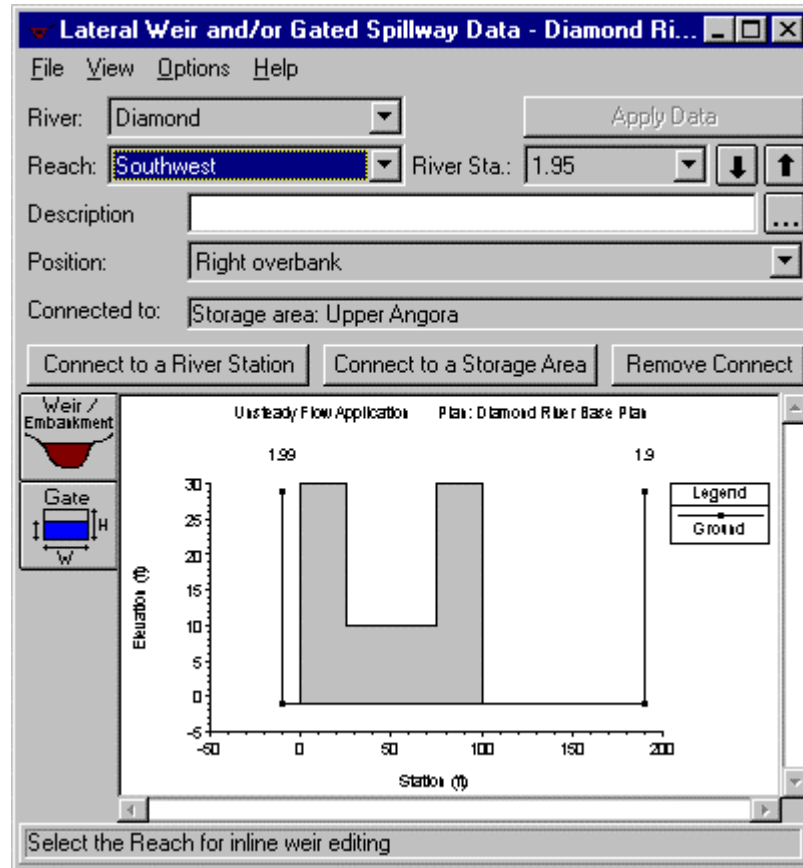


Figure 17.6 Lateral Weir Data Editor

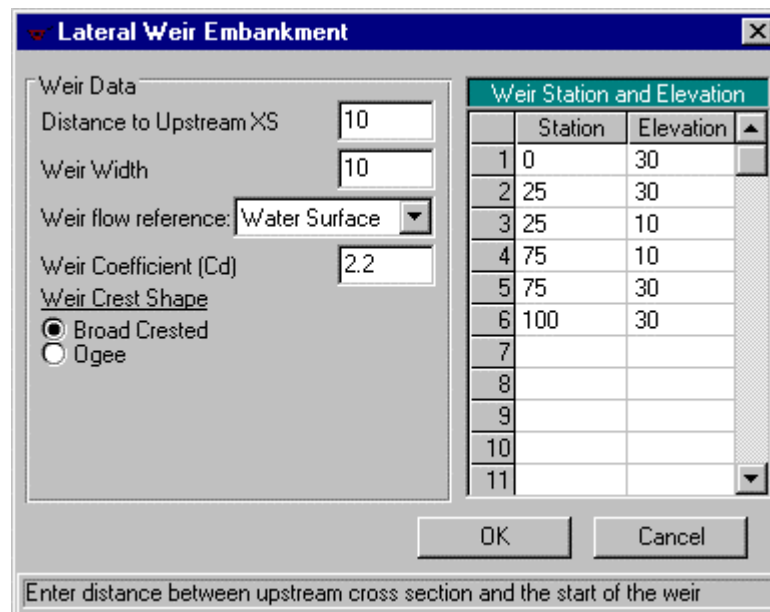


Figure 17.7 Lateral Weir Embankment Window

## Hydraulic Connections

Hydraulic connections are used to connect storage areas to cross sections and to other storage areas.

To make a connection from River Station 1.9 on the Southeast Reach to Eagle Lake, the **Hydraulic Connection** icon was selected from the **Tools** toolbar in the **Geometric Data Editor**. The pointer was placed at cross section 1.9 and the mouse was single clicked. The pointer was then dragged into the storage area titled Eagle Lake and the mouse was double clicked. This brought up a window asking for an identifier for the hydraulic connection and the name “RS 1.9 to Eagle” was entered.

Next, the **Hydraulic Connection** icon under the **Editors** toolbar was selected. This brought up the **Connection Data Editor** shown in Figure 17.8. Under the Hydraulic Connection pull-down menu “RS 1.9 to Eagle” was selected. Then the **Weir/Embankment** icon was selected. A broad crested weir 10 feet wide using the energy grade method was entered. The weir is 100 feet long with an elevation of 17 feet.

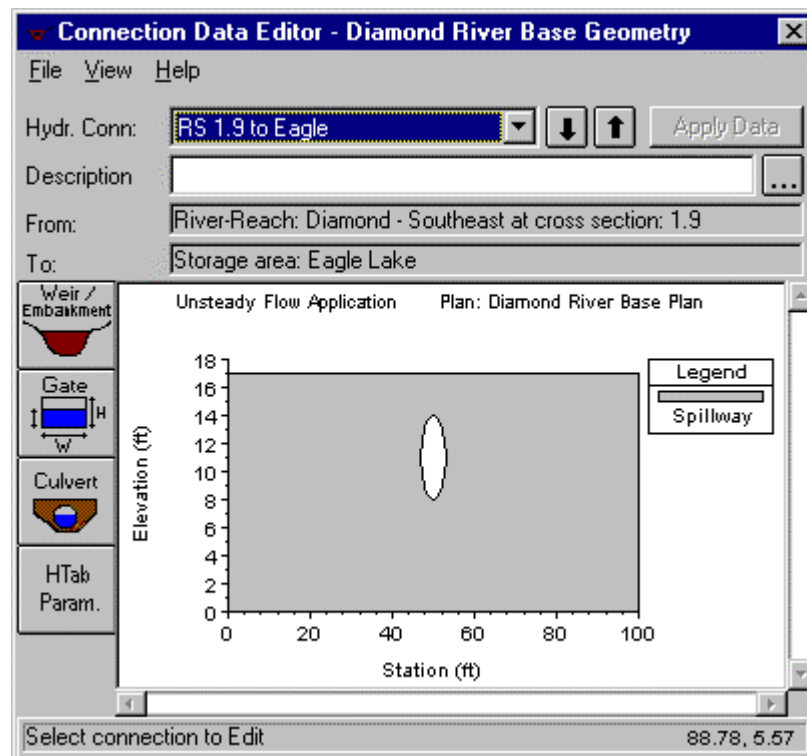


Figure 17.8 Connection Data Editor for RS 1.9 to Eagle

An opening in the weir was placed along the centerline of the weir by selecting the **Culvert** icon on the **Connection Data Editor**. This selection displayed the **Culver Data Editor** shown on Figure 17.9. A circular culvert



with a diameter of 6 feet and an invert elevation of 8 feet was entered for this hydraulic connection. Additional parameters entered are shown in Figure 17.9.

Two additional hydraulic connections (Pyramid to Eagle and Upper to Lower) were constructed in the same manner to connect one storage area to another. The hydraulic connections were made by clicking the **Hydraulic Connection** icon in the **Tools** toolbar and selecting one storage area with a single click of the mouse and dragging the pointer inside the other storage area and double clicking the mouse.

Centerline Stations		
	Upstream	Downstream
1	50	50
2		
3		
4		
5		

Figure 17.9 Culvert Data Editor for RS 1.9 to Eagle

## Parameters for Hydraulic Tables

Hydraulic structures, such as bridges and culverts, are converted into families of rating curves that describe the structure as a function of tailwater, flow, and headwater. The user can set parameters to define the curves by selecting **HTab Parameters** from the **Bridge and Culvert Editor** or from the **Hydraulic Connection Editor**. For this example, the three culverts in hydraulic connections, the culvert located on the Northwest Reach, and the bridge located on the South Reach may have their parameters adjusted.

For the bridge located on the South Reach the **Bridge/Culvert** icon on the **Editor** toolbar in the **Geometric Data Editor** was selected. In the **Bridge Culvert Data** the South Reach was selected and the **HTab Param** icon was depressed bringing up the **Parameters for Hydraulic Properties Tables** shown in Figure 17.10. Default values were used for the number of points on

a free flow curve, the number of submerged curves, and the number of points on each submerged curve. Limits on the extent of the curves were defined by setting the maximum headwater and tailwater elevations at 19 feet. For additional information on HTAB Parameters refer to Chapter 8 of the User's Manual, Performing an Unsteady Flow Analysis.

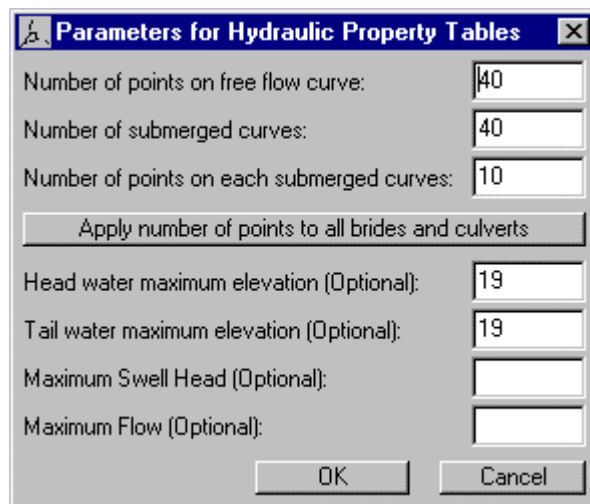


Figure 17.10 Parameter for Hydraulic Property Tables

## Cross Section Table Parameters

In HEC-RAS, cross sections are processed into tables of elevation versus hydraulic properties of areas, conveyances, and storage. The user is required to set an interval for spacing the points in the cross-section tables. In the **Geometric Data Editor** the icon **Htab Parameters** was selected under the **Editors** toolbar. This displays the **Cross Section Table Parameters Window** shown in Figure 17.11. The starting elevation, table increment, and number of points columns are automatically filled by the program but may be changed by the user. The program automatically starts the table for each cross section 1 foot above the channel invert. The program chooses a table increment and number of points by first attempting to use 20 points, and choosing an increment that will put the table up to the top of the cross section. If this results in too large of an increment (greater than 1.0 foot) the program will use a one foot increment and add additional points to get the table to the top of the cross section. The increment for this example was set to 1 for "All Reaches" by highlighting the increment column and selecting the **Set Values** button. The user can set individual values for each cross section if it is deemed necessary. **Warning: Always ensure that your cross section tables encompass the complete range of stages that will be modeled.** If the tables do not go up to a high enough elevation, the program will have to extrapolate during the computations. More often than not, the extrapolation causes instabilities in the results.

**Cross Section Table Parameters**

River:    ☒ Edit Interpolated XS's

Reach:

Selected Area Global Edits

	Reach	River Sta	Chan Min	Starting El	Increment	Num Points (20-100)
1	North	6.0	3	4	1	29
2	North	5.8	2.2	3.2	1	29
3	North	5.6	1.4	2.4	1	29
4	North	5.4	.6	1.6	1	29
5	North	5.2	-.2	.8	1	29
6	North	5.0	-1	0	1	29
7	North	4.8	-1	0	1	29
8	North	4.6	-1	0	1	29
9	North	4.4	-1	0	1	29
10	North	4.2	-1	0	1	29
11	North	4.0	-1	0	1	29
12	Northwest	4.0	-1	0	1	29
13	Northwest	3.8	-1	0	1	29
14	Northwest	3.6	-1	0	1	29

Figure 17.11 Cross Section Table Parameters

## Unsteady Flow Data

The user is required to enter boundary conditions and initial conditions for the system at the beginning of the simulation period. For additional information on unsteady flow data refer to Chapter 8 of the User's Manual, Performing an Unsteady Flow Analysis. The **Unsteady Flow Data Editor**, shown in Figure 17.12, was selected from the main program window under the **Edit** menu.

### Boundary Conditions

Boundary conditions are required for the farthest upstream and downstream cross sections. Upon entering the **Unsteady Flow Data Editor** the farthest upstream and downstream cross sections will be located in the boundary conditions column. In this case, the user will see River Station 6.0 on the North Reach and River Station 0.0 on the South Reach. Boundary conditions are set by highlighting the adjacent cell under **Boundary Condition Type**. When a cell is highlighted, not all boundary conditions are available. The program will automatically gray out all irrelevant boundary condition types. Internal boundary conditions may be added by selecting the desired cross section in the **Unsteady Flow Data Editor** and pressing the **Add a Boundary Condition Location** button.

**Unsteady Flow Data**

File Options Help

Boundary Conditions Initial Conditions Apply Data

Select Location for Boundary Condition

River: Diamond

Reach: North River Sta.: 6.0 Add a Boundary Condition Location

Boundary Condition Types

Stage Hydrograph	Flow Hydrograph	Stage and Flow Hydr.	Rating Curve
Normal Depth	Lateral Inflow Hydr.	Uniform Lateral Inflow	Groundwater Interflow
T.S. Gate Openings	Elev Controlled Gates	Internal Obs. Stage	Intern. Obs. Stage + Flow

	River	Reach	RS	Boundary Condition Type
1	Diamond	North	6.0	Flow Hydrograph
2	Diamond	South	0.0	Normal Depth

Storage Area and Hydraulic Connections: Storage Area: Pyramid L2 Add a Boundary Condition Location

	Storage Cell or Connection	Boundary Condition Type

Figure 17.12 Unsteady Flow Data Editor

## Upstream Boundary Condition

For River Station 6.0 a flow hydrograph was selected by highlighting the cell and pressing the **Flow Hydrograph** button. The window for a **Flow Hydrograph** is shown in Figure 17.13. For this example the flow hydrograph was manually entered by selecting the **Enter Table** radio button. The **Data Time Interval** was set to three hours and the radio button **Use Simulation Time** was selected. The **Use Simulation Time** option starts the hydrograph at the beginning of the simulation time window, which is discussed in an upcoming section. The hydrograph was then manually entered with a baseflow of 100 cfs and a floodwave that peaked at 5000 cfs.

**Flow Hydrograph**

River: Diamond Reach: North RS: 6.0

☐ Read from DSS before simulation Select DSS file and Path

File:

Path:

☒ Enter Table Data time interval: 3 Hour

Select/Enter the Data's Starting Time Reference

☒ Use Simulation Time: Date: 01/01/1990 Time: 0800

☐ Fixed Start Time: Date:  Time:

No. Ordinates Interpolate Missing Values Del Row Ins Row

Hydrograph Data			
	Date	Simulation Time	Flow
		(hours)	(cfs)
1	01Jan1990 0800	00:00	100
2	01Jan1990 1100	03:00	100
3	01Jan1990 1400	06:00	100
4	01Jan1990 1700	09:00	100
5	01Jan1990 2000	12:00	100
6	01Jan1990 2300	15:00	100
7	02Jan1990 0200	18:00	100
8	02Jan1990 0500	21:00	100
9	02Jan1990 0800	24:00	100
10	02Jan1990 1100	03:00	100

Time Step Adjustment Options ("Critical" boundary conditions)

☐ Monitor this hydrograph for adjustments to computational time step

Max Change in Flow (without changing time step):

OK Cancel

Figure 17.13 Flow Hydrograph for Upstream Boundary Condition

The user may also select to read hydrograph data from a DSS file. To do this the user presses the button **Select DSS file and Path**. When this button is pressed a DSS file and pathname selection screen will appear as shown in Figure 17.14. The user first selects the desired DSS file by using the browser button at the top. Once a DSS file is selected, a list of all of the DSS pathnames within that file will be displayed in the table. The user can use the pathname filters to reduce the number of pathnames shown in the table. When the desired DSS pathname is found the user closes the window and the filename and pathname will be recorded in the **Flow Hydrograph Window**.

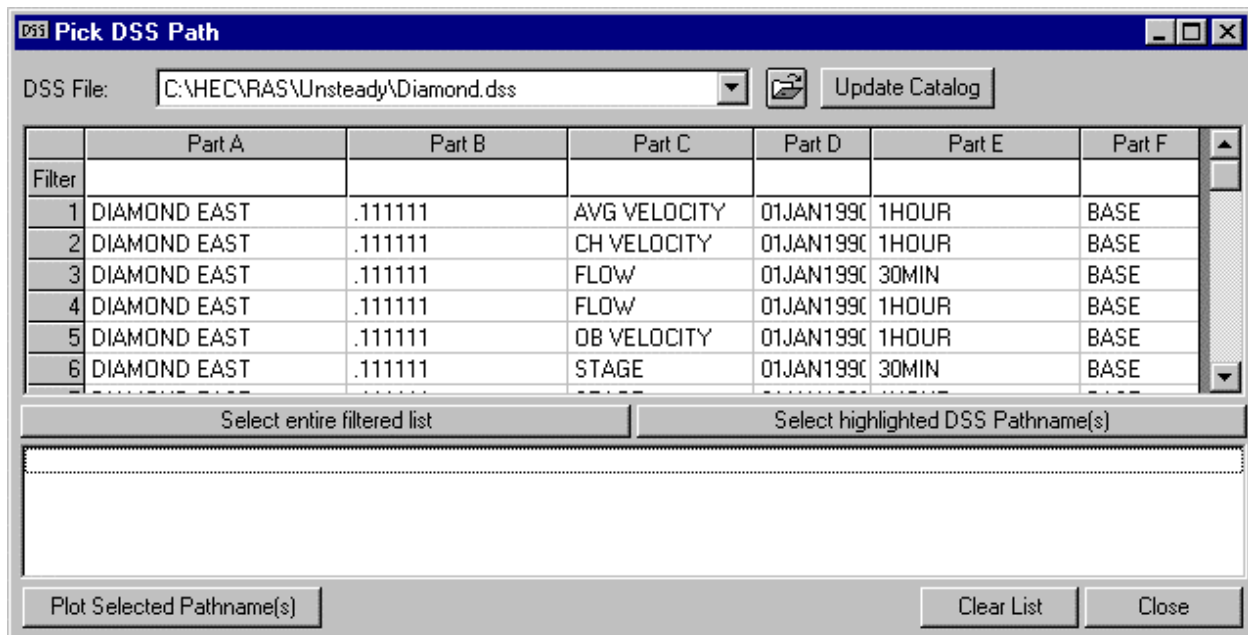


Figure 17.14 DSS Pathname and Filename

## Downstream Boundary Condition

For River Station 0.0 of the South Reach the boundary condition selected was normal depth. Again, the corresponding cell for that particular river station was highlighted and the **Normal Depth** button was selected. The window shown in Figure 17.15 is then displayed and a value of 0.0000947 was entered. This method requires the user to enter a friction slope for the reach in the vicinity of the boundary condition, the slope of the water surface is often a good estimate of the friction slope.

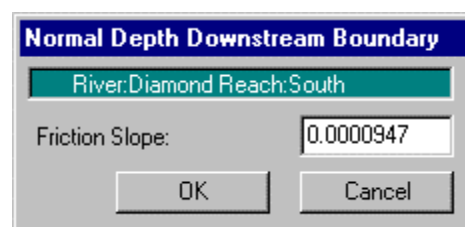


Figure 17.15 Normal Depth for Downstream Boundary

## Initial Conditions

Initial conditions consist of flow and stage information at each of the cross sections, as well as elevations for any storage areas defined in the system. Data for initial conditions is established by selecting the **Initial Conditions** tab on the **Unsteady Flow Data Editor**. After selecting the tab, Figure 17.16 will be displayed.

**Unsteady Flow Data**

File Options Help

Boundary Conditions Initial Conditions Apply Data

Initial Flow Distribution Method

☐ Use a Restart File      Filename:

☒ Enter Initial flow distribution

Locations of Flow Data Changes

River:

Reach:       River Sta.:       Add A Flow Change Location

	River	Reach	RS	Initial Flow
1	Diamond	North	6.0	100
2	Diamond	Northwest	4.0	70
3	Diamond	Northeast	3.9999	30
4	Diamond	Southeast	2.0	20
5	Diamond	Southwest	2.0	60
6	Diamond	South	2.0	80
7	Diamond	West	1.0	10
8	Diamond	East	1.0	10

Initial Elevation of Storage Cells

Storage Cell	Initial Elevation
Pyramid Lake	11
Eagle Lake	10
Lower Angora	12
Upper Angora	10

**Figure 17.16 Unsteady Flow Data Initial Conditions**

Flow data was entered for each reach so the program could perform a steady-flow backwater run to compute the corresponding stages at each cross section.

As shown on Figure 17.16, the North Reach has an initial flow of 100 cfs, corresponding to the baseflow of the hydrograph entered as the upstream boundary condition. The other various reaches then split the 100 cfs of flow down to the South Reach where the flow combines to 80 cfs. In addition to flow data, initial elevation for each storage area is needed. Initial elevations were set to 11, 10, 12, and 10 feet for Pyramid Lake, Eagle Lake, Lower Angora, and Upper Angora, respectively. The data was then saved in the **Unsteady Flow Data Editor** and the window was closed.

## Unsteady Flow Analysis

After the geometry and unsteady flow data have been completed an unsteady flow analysis may be initiated. Under the main program window **Unsteady Flow Analysis** was selected under the **Run** menu. This will display the **Unsteady Flow Analysis Window** shown in Figure 17.17. A plan was defined by selecting the Geometry File “Diamond River Base Geometry” and the Unsteady Flow File “Unsteady Flow.” Under the **File** menu, **Save Plan As** was selected and the plan title was entered as “Diamond River Base Plan.” After entering the title of the plan a short identifier was entered as “Base” and the plan was saved.

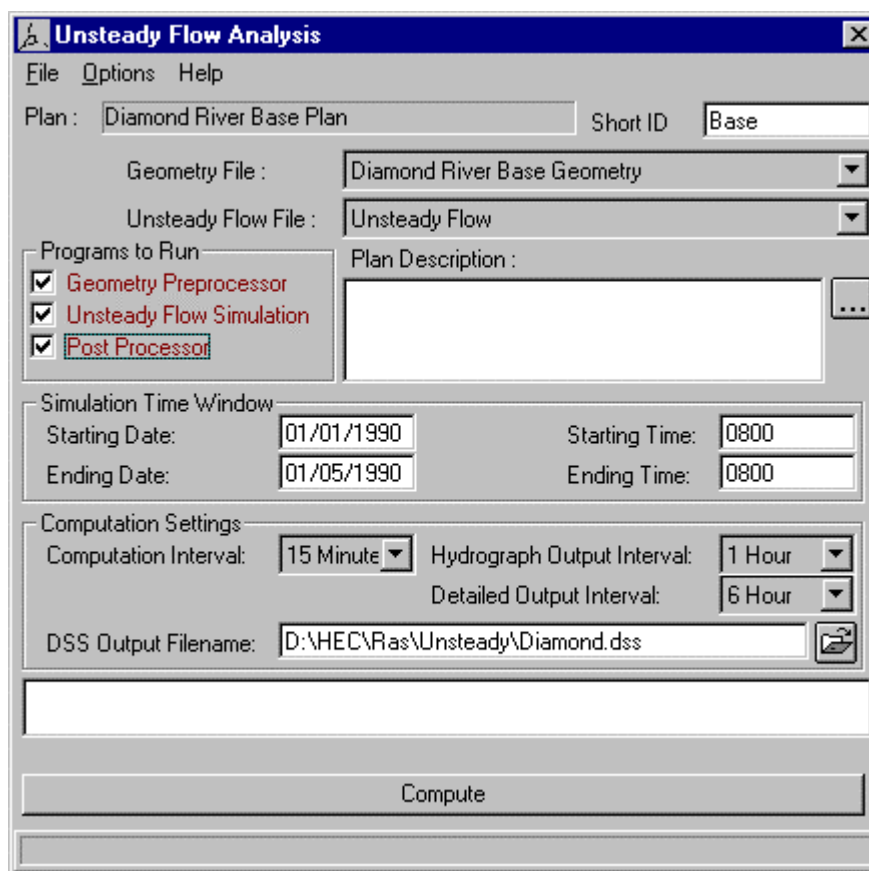


Figure 17.17 Unsteady Flow Analysis Window

### Simulation Time Window

The simulation time window, located on Figure 17.17, requires a beginning and ending date and time for simulation. The date must have a four digit year and can be entered in either of the two following formats: 01Jan1990 or 01/01/1990. For this application the date was saved in the latter format. The time field is entered in military time (i.e. 1 p.m. is entered as 1300). For this application the simulation time began and ended at 0800.



## Computation Settings

The computation settings in the **Unsteady Flow Analysis Window** contain the following: the computational interval; hydrograph output interval; instantaneous profiles interval; and the name and path of the output DSS file. For this example the computation interval was set to 15 minutes. The computation interval should be small enough to accurately describe the rise and fall of the floodwave. The hydrograph output interval was set to 1 hour. This interval defines the output of computed stage and flow hydrographs written to HEC-DSS. The detailed output interval was set to 6 hours, specifying the interval at which detailed hydraulic output will be computed by the post processor. It is suggested that this interval remain fairly large to reduce the amount of post processing and storage required. The path selected for the output to DSS was “C:\HEC\RAS\Unsteady\Diamond.dss.”

## Location of Stage and Flow Hydrographs

The user has the option of specifying locations to have hydrographs computed and available for display. The user may select individual cross sections, groups of cross sections, or entire reaches. From the **Options** menu on the **Unsteady Flow Analysis Window**, the **Stage and Flow Output Locations** option was selected, displaying Figure 17.18. For this example “All Reaches” was selected. This option will compute hydrographs at every cross section in the data set. If the user is working with an extremely large data set, computation time and data storage can be reduced by only selecting the most essential cross sections for output.

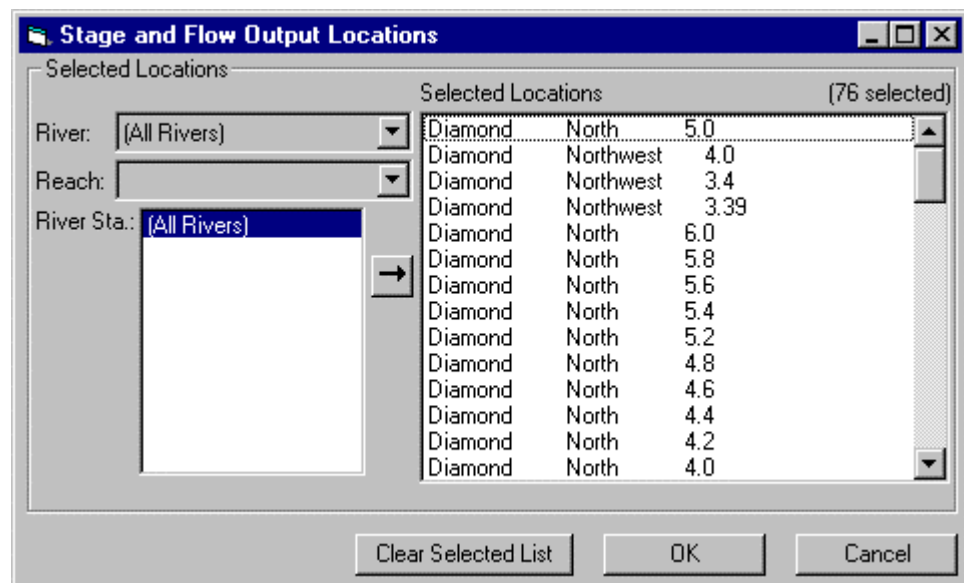


Figure 17.18 Stage and Flow Output Locations

## Unsteady Flow Simulation

There are three components used in performing an unsteady flow analysis within HEC-RAS: the geometric data preprocessor (HTAB); the unsteady flow simulator (UNET); and an output post processor.

### Geometry Pre-processor (HTAB)

The geometry pre-processor is used to speed up the unsteady flow calculations by processing the geometric data into a series of hydraulic property tables and rating curves. It is highly recommended and illustrated in this example that the user run the geometry pre-processor and examine hydraulic output for anomalies before running the unsteady flow simulator and the post-processor.

The **Geometry Pre-processor** box under “Programs to Run” was selected in the **Unsteady Flow Analysis Window**, Figure 17.17. The **Unsteady Flow Simulation** and **Post-Processor** boxes were unselected (no checkmarks) and the **Compute** button was depressed. After the geometry pre-processor finished processing the data, **Hydraulic Property Plots** was selected from the **View** menu on the main program menu. Figure 17.19 displays a typical cross section plot of hydraulic properties for this example. Cross sections are processed into tables of elevations versus hydraulic properties of areas, conveyances, and storage. Viewing these plots for anomalies is a good diagnostic tool to search for cross sections with irregular or erroneous geometry.

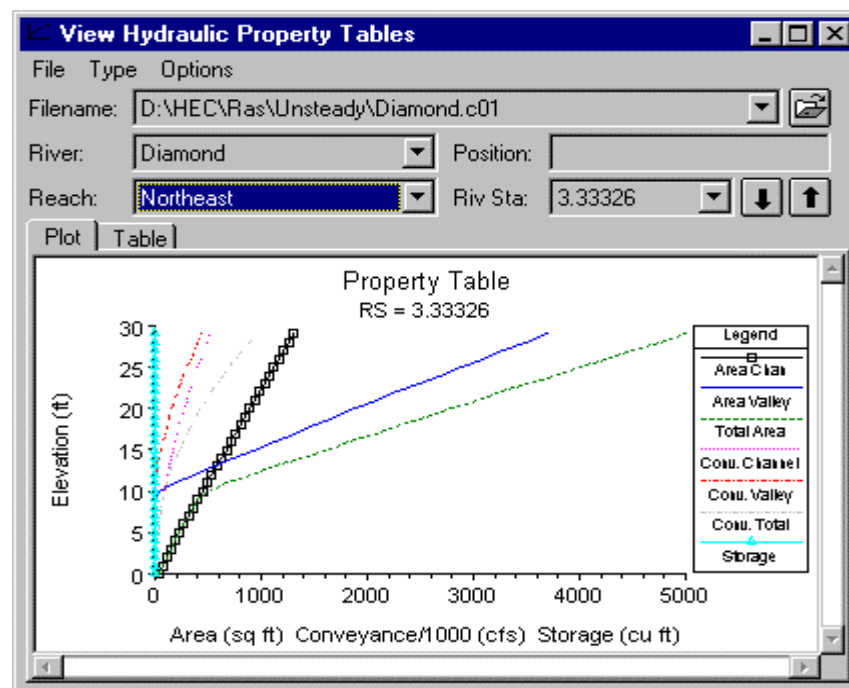
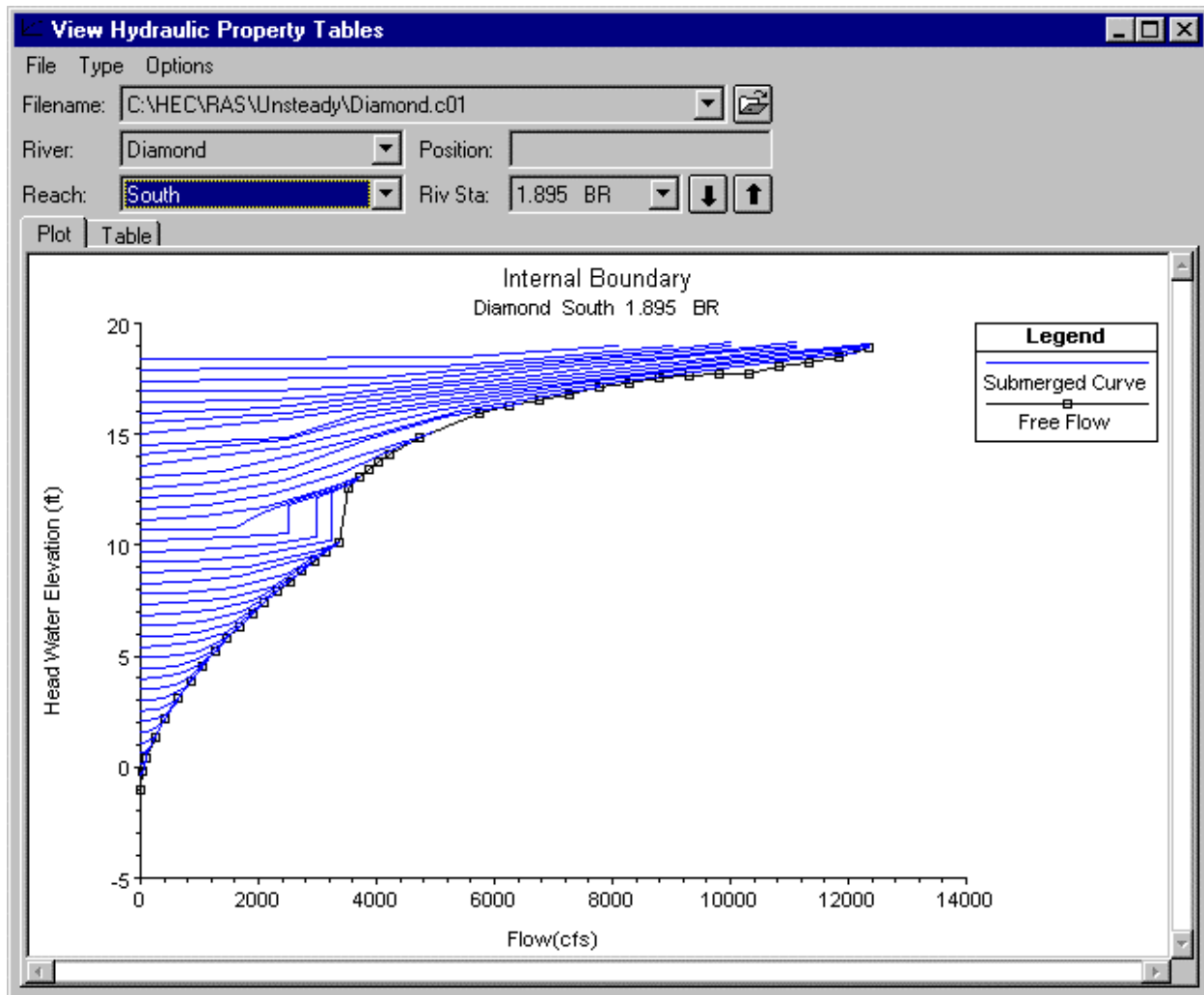


Figure 17.19 Hydraulic Properties for a Cross Section

The geometry pre-processor evaluates hydraulic structures such as bridges and culverts and relates the structures as a function of tailwater, flow, and headwater. From the **Type** menu in the **Hydraulic Properties Tables**, **Internal Boundaries** was selected. Next, the South Reach was selected displaying Figure 17.20, the family of rating curves for the bridge located at RS 1.895. On the plot the free flow rating function describes the flow if tailwater submergence does not occur, such as free flow over a weir. Above the free-flow rating function is a family of submerged flow rating curves, one for each tailwater elevation.



**Figure 17.20 Family of Rating Curves for Bridge Located on the South Reach**

As shown in Figure 17.20 there is a transitional area to left of the free flow curve between 10 and 12 feet of elevation where the lines of constant tailwater extend vertically. This occurs because at this water elevation flow begins to contact the upstream side of the bridge, causing backwater. This is a transition zone where free surface flow changes to orifice flow. This type of

flow is unpredictable because the flow is changing from free surface flow to a “sluice gate type” of pressure flow and then possibly to a full flowing orifice flow.

## Unsteady Flow Simulation and the Post-Processor

After the hydraulic property tables were examined the **Geometry Pre-processor** box was unselected on the **Unsteady Flow Analysis Window** and the **Unsteady Flow Simulation** and **Post-Processor** boxes were selected. Then the **Compute** button was depressed.

The output from the simulation can be viewed from many different tables and graphs. The most interesting output for this example is found by viewing the stage and flow hydrographs. From the main program window, **View** and then **Stage and Flow Hydrograph** was selected. By selecting the **Type** menu on the **Stage and Flow Hydrograph Window** the user can view stage and flow hydrographs for cross sections, bridges, culverts, inline weirs, lateral weirs, storage areas, and hydraulic connections.

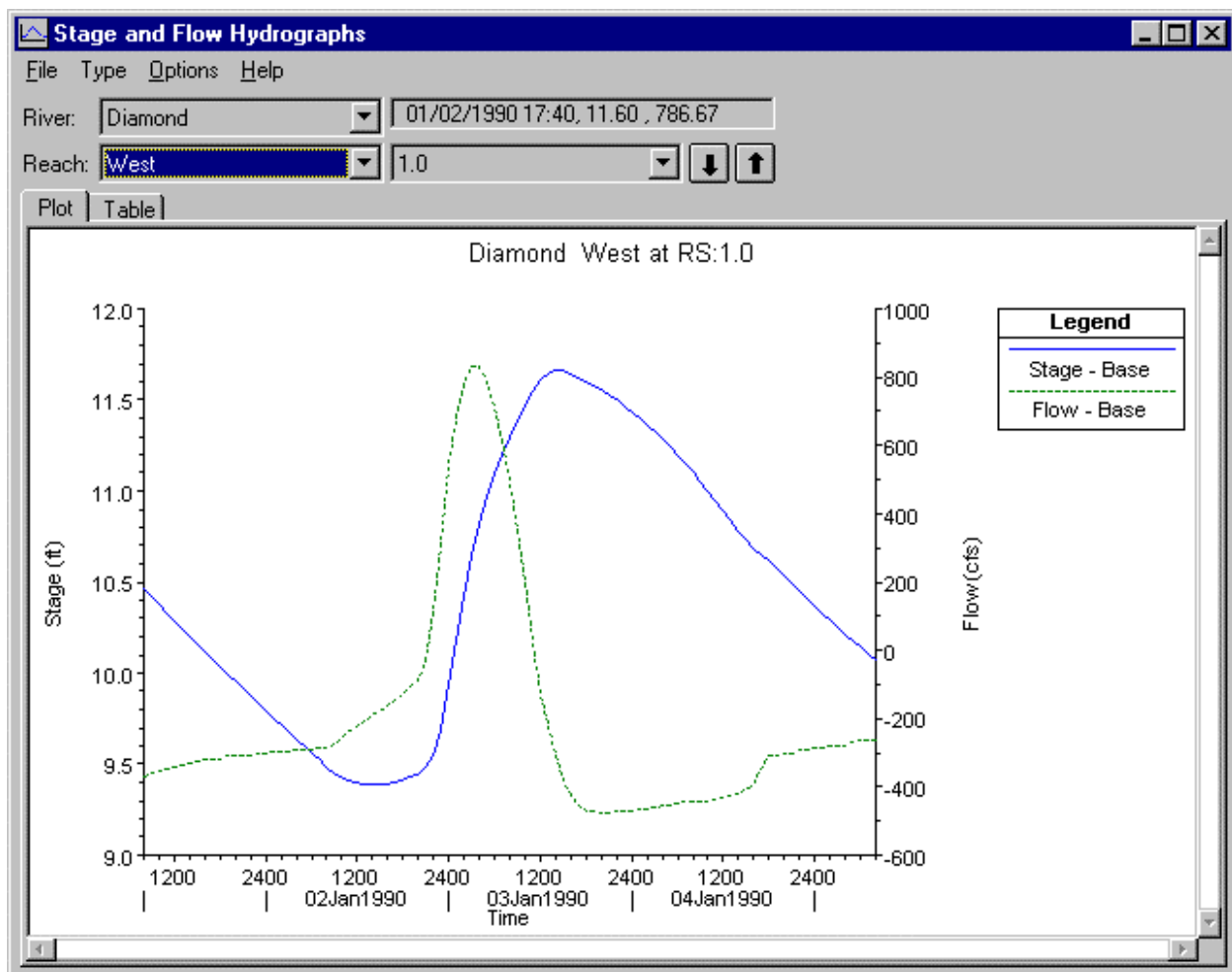
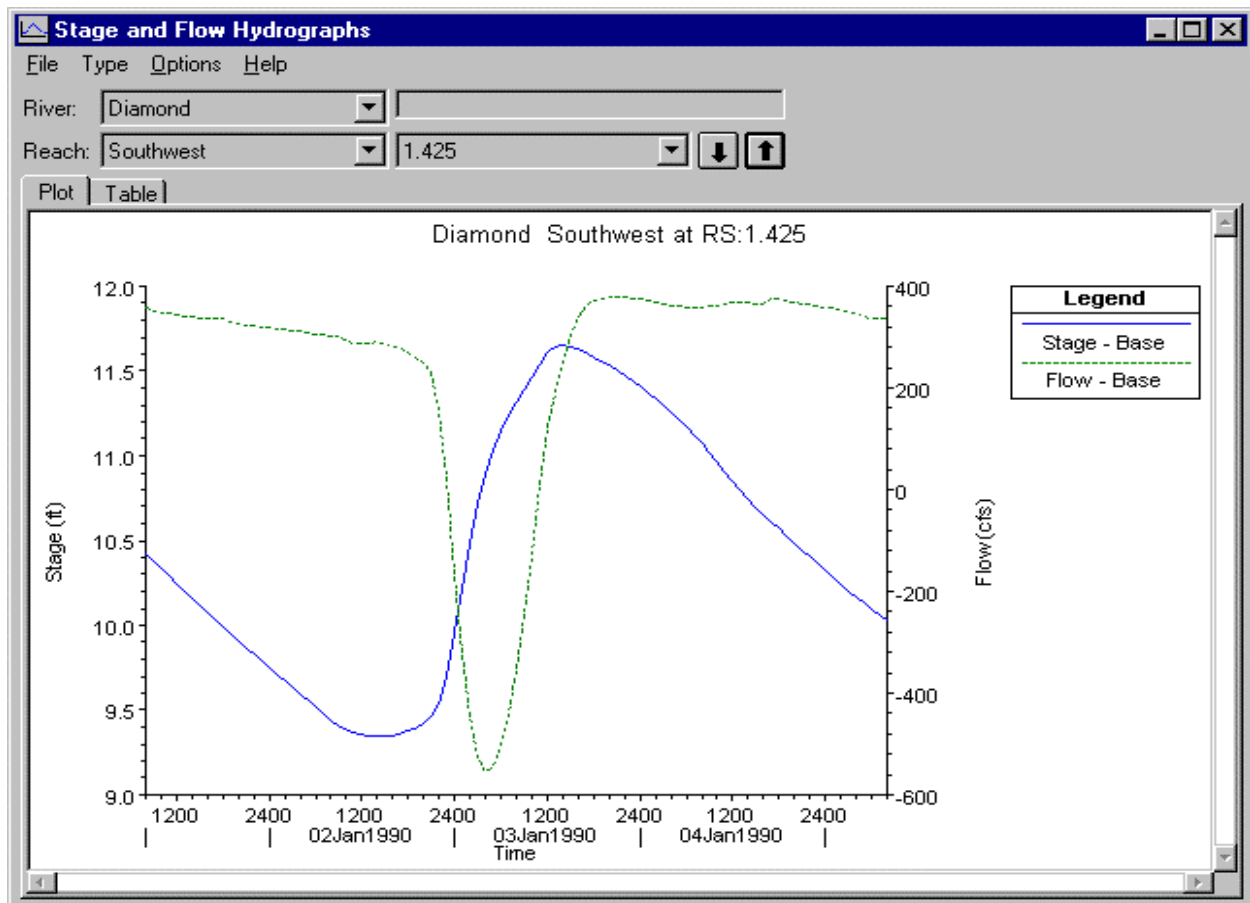


Figure 17.21 Stage and Flow Hydrograph for the West Reach

First, the “Cross Section” option was selected from the **Type** menu and the reach was set to West, displaying Figure 17.21. As seen from the figure, the flow was initially negative, denoting that at the beginning of the simulation time flow was moving away from Pyramid Lake. As the floodwave progressed the flow changed to positive, implying a reversal in the direction of flow towards Pyramid Lake. After the floodwave passed, the direction of flow returned to flowing away from Pyramid Lake. The stage and flow can also be viewed as tabular output by selecting the **Table** tab located on the **Stage and Flow Hydrograph Window**. The output will be displayed in the increment set on the hydrograph output interval located in the **Unsteady Flow Analysis Window**.

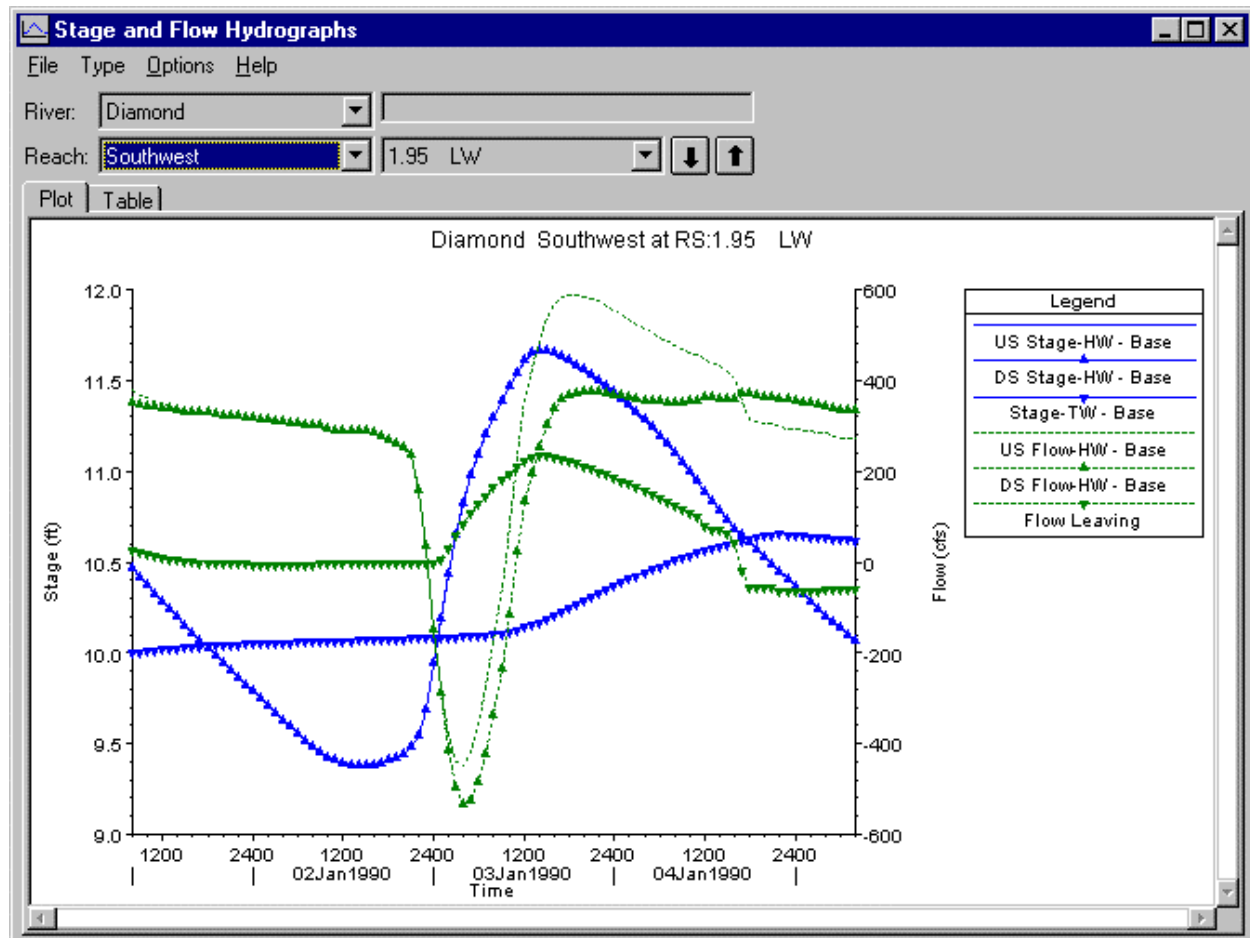


**Figure 17.22 Stage and Flow Hydrograph for Southwest Reach**

The geometric data set was constructed with relatively no slope to emphasize the ability of RAS to model unsteady flow, including flow reversals. As seen in Figure 17.22 for the Southwest Reach, the flow reverses direction during the peak of the floodwave. This flow reversal occurs because water is diverted to the West Reach and to the lateral weir on the Southwest Reach. These diversions decrease the flow in the Southwest Reach compared to the flow in the Southeast Reach. The discrepancy in flows between the two reaches causes a significant difference in water surface elevations at the

Lower Junction. The difference in water surface elevation forces water to move upstream on the Southwest Reach during the floodwave.

Next, the **Lateral Weir** option was selected from the **Type** menu and the reach was set to Southwest, displaying Figure 17.23. As shown in Figure 17.23, the lateral weir is affected by the flow reversal on the Southwest Reach. Additionally, the tailwater stage for the lateral weir rises above the headwater stage twice during simulation, causing flow to enter the reach from the lateral weir.

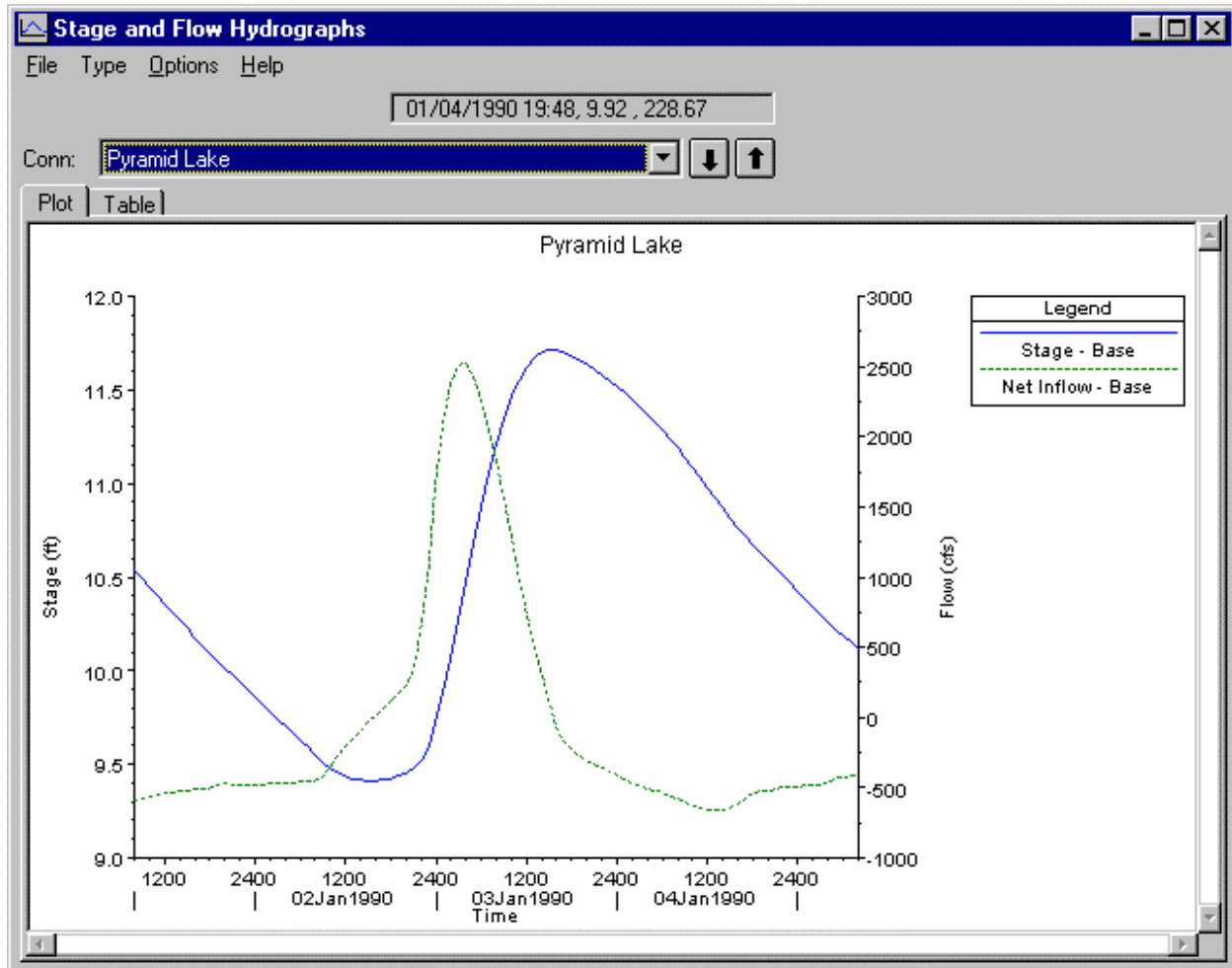


**Figure 17.23 Stage and Flow Hydrograph for Lateral Weir Located on the Southwest Reach**

In addition to viewing output directly from the **Stage and Flow Hydrograph Window** the user can open the **Geometric Data Editor** and select to view output by clicking on the desired feature. From the **Geometric Data Editor** the storage area "Pyramid" was selected by clicking on it with the mouse. **Plot Stage and Flow Hydrograph** was selected, displaying Figure 17.24.

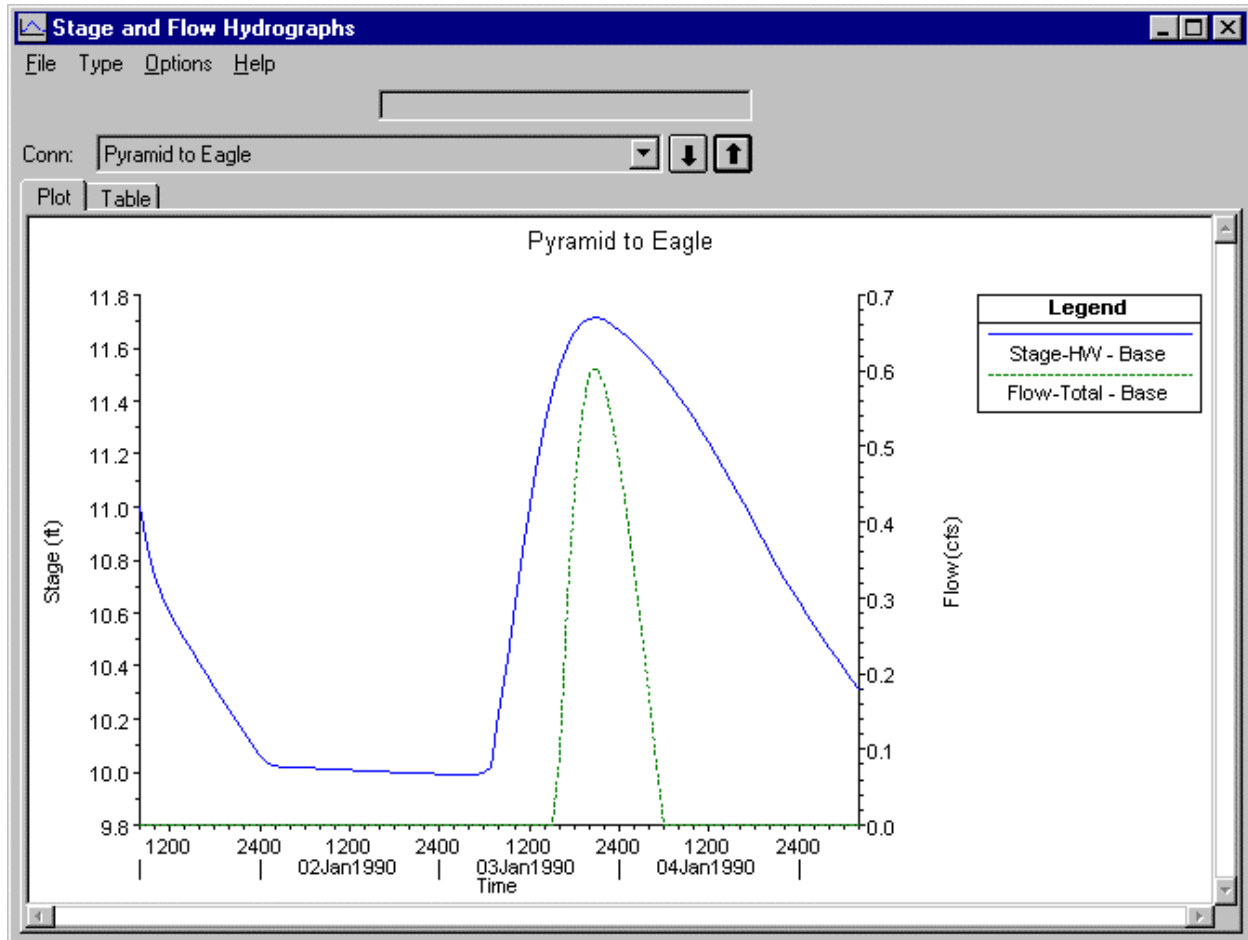
As shown in Figure 17.24, the stage of the storage area steadily decreases because flow is leaving the storage area until the floodwave passes through,

causing the stage to increase. The stage of the storage area then begins to decrease again after the floodwave.



**Figure 17.24 Stage and Flow Hydrograph for Pyramid Lake**

Finally, from the **Geometric Data Editor** the hydraulic connection “Pyramid to Eagle” was selected, displaying Figure 17.25. Initially, there is no flow in this connection. This occurs because the stage is below the low invert elevation of the culvert at 11.5 feet. When the stage increases to over 11.5 feet the flow rate steadily increases, as shown on Figure 17.25.



**Figure 17.25 Stage and Flow Hydrograph for Hydraulic Connection – Pyramid to Eagle**

## Summary

The concept of unsteady flow analysis within a network of channels and storage areas was discussed. This example demonstrates the ability of HEC-RAS to route a hydrograph through a network of channels containing junctions, culverts, bridges, storage areas, lateral weirs, and hydraulic connections.

Unsteady flow analysis can be extremely difficult compared to steady flow analysis because input parameters can cause instabilities in calculations. Instabilities can cause the program to fail to converge on a solution. It is highly recommended that the user have experience with unsteady flow modeling. Refer to Chapter 8 of the User's Manual for additional information on unsteady flow analysis.